

CIVIL ENGINEERING

ESE SUBJECTWISE CONVENTIONAL SOLVED PAPER-II

23
YEARS
SOLUTION

COMPLETE SOLUTIONS WITH EXPLANATIONS | THOROUGHLY REVISED AND UPDATED

CIVIL ENGINEERING

ESE CONVENTIONAL SOLUTION

PAPER-II

FROM (1995-2017)

UPSC Engineering Services Examination, GATE,
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(BHEL, NTPC, NHPC, DRDO, SAIL, HAL, BSNL, BPCL, NPCL, etc.)



IES MASTER PUBLICATION

Office : F-126, (Lower Basement) Katwaria Sarai, New Delhi - 110 016

Web : www.iesmasterpublications.com | Phone : 011-26522064 | Mobile : 8130909220, 9711853908



IES MASTER Publication

F-126, Katwaria Sarai, Lower Basement New Delhi-110016

Phone : 011-41013406, Mobile : 8130909220, 9711853908

Web : www.iesmasterpublications.com E-mail : info@iesmaster.org

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Second Edition : 2017



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Preface

It is an immense pleasure to present topic wise previous years solved paper of Engineering Services Exam. This booklet has come out after long observation and detailed interaction with the students preparing for Engineering Services Exam and includes detailed explanation to all questions. The approach has been to provide explanation in such a way that just by going through the solutions, students will be able to understand the basic concepts and will apply these concepts in solving other questions that might be asked in future exams.

Engineering Services Exam is a gateway to a immensely satisfying and high exposure job in engineering sector. The exposure to challenges and opportunities of leading the diverse field of engineering has been the main reason for students opting for this service as compared to others. To facilitate selection into these services, availability of arithmetic solution to previous year paper is the need of the day. Towards this end this book becomes indispensable.

Mr. Kanchan Kumar Thakur
Director–IES Master

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Environmental Engineering

Syllabus

(a) Water Supply Engineering

Sources of supply, yields, design of intakes and conductors; Estimation of demand; Water quality standards; Control of Water-borne diseases; Primary and secondary treatment, detailing and maintenance of treatment units; Conveyance of treatment units; Conveyance and distribution systems of treated water leakages and control; Rural water supply; Institutional and industrial water supply.

(b) Waste Water Engineering:

Urban rain water disposal; Systems of sewage collection and disposal; Design of sewers and sewerage systems, pipping; Characteristics of sewage and its treatment, Disposal of products of sewage treatment. Stream flow rejuvenation Institutional and industrial sewage management; Plumbing Systems; Rural and semi-urban sanitation.

(c) Solid Waste Management

Source, classification collection and disposal; Design and Management of landfills.

(d) Air and Noise Pollution and Ecology

Sources and affects of air pollution, monitoring of air pollution; Noise pollution and standards; Ecological chain and balance, Environmental assessment.

IES – 1995

1. *Determine the surface area of a settling tank for 0.5 m³/sec design flow, using the design overflow rate as 32.5 m³/day/m². Find the depth of the clarifier for the overflow rate and detention time of 95 mins. Assume, length-to-width ratios for settling tank between 2 : 1 and 5 : 1, and length not to exceed 100 m. Recommend the dimensions of the tank.*

[20 Marks]

Sol. Data given:

$$\text{Discharge, } Q = 0.5 \text{ m}^3/\text{s} = 43,200 \text{ m}^3/\text{day}$$

$$\text{Overflow rate} = 32.5 \text{ m}^3/\text{day}/\text{m}^2$$

$$\text{Detention time, } t = 95 \text{ mins}$$

$$\text{Surface area of settling tank, } A = \frac{\text{Discharge}}{\text{Overflow Rate}} = \frac{43,200}{32.5} = 1329.231 \text{ m}^2$$

$$\text{Volume of tank, } V = Q \times t = 0.5 \times 95 \times 60 = 2850 \text{ m}^3$$

$$\text{Depth of clarifier, } H = \frac{V}{A} = \frac{2850}{1329.231} = 2.144 \text{ m}$$

Assume $\frac{L}{B} = 2$

$$\therefore L \times B = A = 1329.231$$

$$\Rightarrow 2B^2 = 1329.231$$

$$\Rightarrow B = 25.78 \text{ m} \text{ \& } L = 51.56 \text{ m}$$

Adopt, Length, $L = 52 \text{ m}$

Width, $B = 26 \text{ m}$

and by providing a freeboard of 0.3 m

$$\begin{aligned} \text{Adopt depth of tank} &= 2.144 + 0.3 = 2.444 \text{ m} \\ &= 2.5 \text{ m} \end{aligned}$$

2. *The treated domestic sewage of a town is to be discharged in a natural stream. Calculate the percentage purification required in the treatment plant with the following data ;*

$$\text{Population} = 50,000,$$

$$\text{B.O.D. contribution per capita} = 0.07 \text{ kg/day},$$

$$\text{B.O.D. of stream on U/S Side} = 3 \text{ mg/lit},$$

Permissible maximum B.O.D. of stream on

$$\text{D/S Side} = 5 \text{ mg/lit},$$

$$\text{Dry weather flow of sewage} = 140 \text{ liters per capita per day},$$

$$\text{Minimum flow of Stream} = 0.13 \text{ m}^3/\text{sec}.$$

Explain graphically the process of self purification of natural waters when sewage is discharged therein.

[15 Marks]

Sol.

$$\text{Population} = 50,000$$

$$\text{BOD contribution per capita} = 0.07 \text{ kg/day}$$

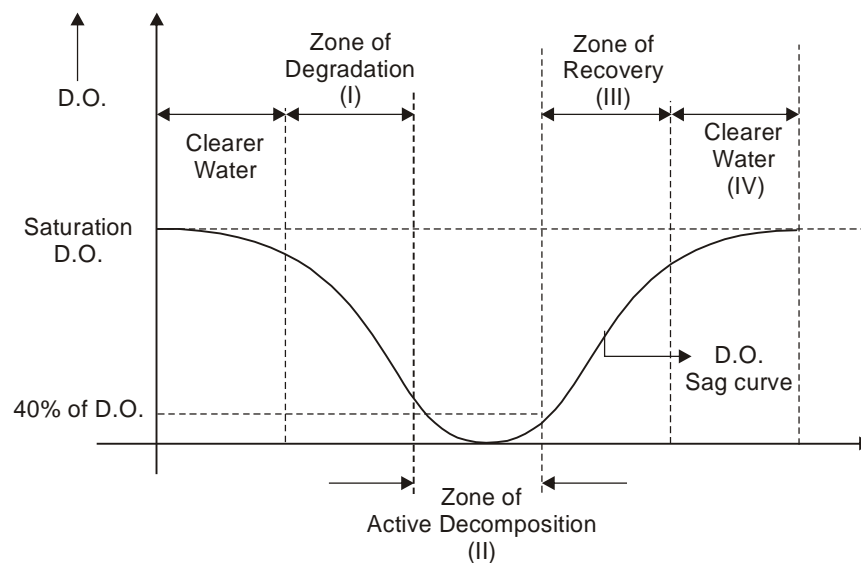
$$\text{Total BOD of raw sewage per day} = 0.07 \times 50,000 = 3500 \text{ kg/day}$$

$$\begin{aligned} \text{Sewage discharge, } Q_s &= \frac{140 \times 50000}{86400} = 81.02 \text{ lit/sec.} \\ &= 0.081 \text{ m}^3/\text{s} \end{aligned}$$

$$\text{BOD of mix} = 5 \text{ mg/l}$$

$$\begin{aligned} \text{BOD of river, } C_R &= 3 \text{ mg/l} \\ \text{Let BOD of treated sewage} &= C_S \\ \therefore \text{BOD of mix} &= \frac{C_R Q_R + C_S Q_S}{Q_R + Q_S} \\ \Rightarrow 5 &= \frac{3 \times 0.13 + C_S \times 0.081}{0.13 + 0.081} \\ \Rightarrow C_S &= 8.211 \text{ mg/l} \\ \text{BOD of untreated sewage} &= \frac{3500 \times 10^6 \text{ mg/d}}{50000 \times 140 \text{ L/d}} \\ &= 500 \text{ mg/L} \\ \text{Percentage purification required} &= \frac{5.00 - 8.21}{500} \times 100 \\ &= 98.36 \% \end{aligned}$$

Zones of pollution in River stream



The Process of self purification of natural waters can be divided into four zones which are described as follows :

Zone of Degradation

- It is found upto certain distance downstream of the point at which sewage is discharged into the river.
- In this algae dies but the fish survives.
- Water becomes dark and turbid.
- DO reduces to upto 40% of saturation.

Zone of Active Decomposition

- It is a zone of heavy pollution.
- Water becomes darker than zone of degradation.
- DO may even fall to zero.
- Fish will disappear.
- At upper ends, anaerobic bacteria will replace aerobic bacteria. Hence, anaerobic conditions set in and thus gases like CH_4 , H_2S , CO_2 will be evolved and ugly scum forms on the surface.
- At the end of this zone DO concentration will reach upto 40% of the saturation DO.

Zone of Recovery

- BOD falls down.
- Organic matter will produce nitrate, sulphate, phosphate, carbonate, etc.
- D.O. content rises above 40% of the saturation value

Zone of Clear Water

- DO will rise upto its saturation value, but the pathogens may remain.

3. Consider the case where a noise level of 90 dBA exists for five minutes and is followed by a reduced noise level of 60 dBA for 50 mins. What is the equivalent continuous equal energy level (L_{eq}) for the 55-minute period? Assume a five-minute sampling period. Write the concepts of L_{eq} .

[15 Marks]

Sol. Equivalent continuous equal energy level,

$$L_{eq} = 10 \log \sum_{i=1}^{i=n} (10)^{L_i/10} \times t_i$$

where, n = total number of sound samples = 2

L_i = the noise level of any i^{th} level, $L_1 = 90\text{dB}$, $L_2 = 60\text{dB}$

t_i = time duration of i^{th} sample, expressed as fraction of total

$$\text{sample time; } t_1 = \frac{5}{55} ; t_2 = \frac{50}{55}$$

$$\begin{aligned} \therefore L_{eq} &= 10 \times \log \left[(10)^{\frac{90}{10}} \times \frac{5}{55} + (10)^{\frac{60}{10}} \times \frac{50}{55} \right] \\ &= 79.629 \text{ dBA} \end{aligned}$$

The concept of L_{eq}

L_{eq} is defined as the constant noise level, which over a given time, expands the same amount of energy, as is expanded by the fluctuating levels over the same time.

L_{eq} noise levels are logarithmic (dB) values and cannot be added directly.

It is common practice to measure noise levels using the A - weighting setting built into all sound level meters. A - weighted measurements give higher weight to frequencies to which human ear is more sensitive and lesser weight to the ones to which human ear is less sensitive. This is done to ensure that the meter is measuring pretty much what a human ear actually hears. Thus A - Weighted measurements are expressed as dBA or dB(A).

IES – 1996

4. *A circular sewer of 45 cm diameter was designed for a town of population 30,000. The sewer was designed to carry 3.5 times of the dry weather flow. What slope should be provided to the sewer when running full? Value of $n = 0.012$ in Manning's equation. Assume other relevant data suitably.*

[10 Marks]

Sol. Assuming water supply per capita = 135/

$$\therefore \text{Total water supplied} = 30000 \times 135 = 4050000 \text{ lit/day} \\ = 0.046875 \text{ m}^3/\text{s}$$

Assuming that 80% of the water supplied to the town appears as sewage,

$$\text{We have average discharge, } Q = 0.8 \times 0.046875 \\ = 0.0375 \text{ m}^3/\text{s}$$

At a peaking factor of 3.5,

$$\text{Design discharge, } Q_d = 3.5 \times 0.0375 = 0.13125 \text{ m}^3/\text{s}$$

$$\text{Hydraulic radius under full flow, } R = \frac{A}{P} = \frac{\pi D^2 / 4}{\pi D} = \frac{D}{4}$$

$$\text{Area of flow, } A = \frac{\pi D^2}{4}$$

Using Manning's equation, we know that

$$Q_d = \frac{1}{n} \times A \times R^{2/3} \times \sqrt{S}$$

$$\Rightarrow 0.13125 = \frac{1}{0.012} \times \frac{\pi D^2}{4} \times \left(\frac{D}{4}\right)^{2/3} \times \sqrt{S}$$

$$\Rightarrow 0.13125 = \frac{1}{0.012} \times \frac{\pi \times 0.45^2}{4} \times \left(\frac{0.45}{4}\right)^{2/3} \times \sqrt{S}$$

$$S = 1.806 \times 10^{-3}$$

or

$$S = 1 \text{ in } 553.78$$

5. *In a continuous flow settling tank, 3.5 m deep and 65 m long, if the flow velocity of water observed is 1.22 cm/s. what size of the particles of specific gravity 2.65 may be effectively removed? Assume temperature 25°C and kinematic viscosity of water as 0.01 cm²/s.*

[10 Marks]



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₹ 775.00

ISBN 978-95-86585-51-8

